

GEOTECHNICAL INVESTIGATION AND ACID SULFATE SOIL ASSESSMENT

FOR

NSW LAND & HOUSING CORPORATION

C/- SMEC AUSTRALIA PTY LIMITED

38-44 John T Bell Drive & 31-35 Matfen Close, Maryland, New South Wales

Report No: 20/3519

Project No: 30615/4133D-G

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DRAWING NO. 20/3519 – BOREHOLE AND PENETROMETER LOCATIONS

NOTES RELATING TO GEOTECHNICAL REPORTS

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1. INTRODUCTION

This report presents the results of a combined Geotechnical Investigation and Acid Sulfate Soil (ASS) assessment carried out by STS Geotechnics Pty Limited (STS) for a proposed new residential development to be constructed at 38-44 John T Bell Drive and 31-35 Matfen Close, Maryland. At the time of writing this report STS were not provided with architectural drawings for the project, however we understand the development will typically comprise the construction of single and double storey residential buildings. The development will not include basement levels. We understand that the site is located within a Class 2 Acid Sulfate Soils area and therefore Council requires an assessment to be undertaken.

The purpose of the investigation was to:

- assess the subsurface conditions over the site,
- provide a Site Classification to AS2870,
- provide recommendations regarding the appropriate foundation system for the site including design parameters,
- comment on soil aggressiveness to buried steel and concrete,
- undertake an ASS assessment, and
- determine if an ASS Management Plan is required.

The investigation was undertaken at the request of SMEC Australia Pty Limited on behalf of NSW Land and Housing Corporation.

Our scope of work did not include a contamination assessment.

2. NATURE OF THE INVESTIGATION

2.1. Fieldwork

The fieldwork consisted of drilling nine (9) boreholes numbered BH1 to BH9, inclusive, at the locations shown on Drawing No. 20/3519. Restricted site access dictated the borehole locations. The boreholes were drilled using a Christie track mounted mini drilling rig owned and operated by STS. Soils and weathered rock were drilled using rotary solid flight augers. Soil strengths were determined by undertaking Dynamic Cone Penetrometer (DCP) tests at each borehole location.

Drilling operations were undertaken by one of STS's senior geologists who also logged the subsurface conditions encountered.

The subsurface conditions observed are recorded on the borehole logs given in Appendix A. An explanation of the terms used on the logs is also given in Appendix A. Notes relating to geotechnical reports are also attached.

2.2. Laboratory Testing

In order to assist with determining the site classification, a shrink swell index test was carried out on a representative sample retrieved from the site.

In order to assess the soils for their aggressiveness, a selected representative soil sample was tested to determine the following:

- pH,
- Sulphate content (SO₄),
- Chloride content (CL), and
- Electrical Conductivity (EC)

Based on field observations, eight (8) soil samples were also selected for laboratory analysis for the Acid Sulfate Soils assessment. The samples were dispatched to Australian Laboratory Services (ALS) for analysis using the Suspension Peroxide Oxidation Combined Acidity and Sulphate (SPOCAS) method. The method allows both a measure of the existing and potential acidity.

Detailed test reports are given in Appendix B.

3. GEOLOGY AND SITE CONDITIONS

The Newcastle-Hunter Area Coastal Quaternary Geology sheet at a scale of 1:100,000 shows the site is underlain by a Pleistocene Terrace. Materials within this formation comprise silt, clay, fluvial sand and gravel.

The site is irregular in shape with an area of approximately 4,784 m². At the time of the fieldwork, weatherboard and brick houses were present. Single and double storey residential dwellings are present in the nearby properties.

Site vegetation comprised grass, trees and shrubs.

The ground surface falls approximately 0.5 metres to the north west.

4. SUBSURFACE CONDITIONS

When assessing the subsurface conditions across a site from a limited number of boreholes, there is the possibility that variations may occur between test locations. The data derived from the site investigation programme are extrapolated across the site to form a geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour regarding the proposed development. The actual condition at the site may differ from those inferred, since no subsurface exploration programme, no matter how comprehensive, can reveal all subsurface details and anomalies.

The subsurface conditions generally consist of topsoil/fill overlying fill, silty sandy clays and silty clays. Topsoil/fill was encountered to depths of 0.1 to 0.3 metres. Silty clay fill underlies the site to depths of 1.2 and 1.9 metres. For the most part, this material seems to be firm to stiff and stiff. Natural silty clays underlie the fill to the depth of drilling, 3.0 metres. The consistency of these materials range between soft to firm and very stiff.

Groundwater was not observed during drilling of the boreholes, however, some moist to wet zones were noted.

5. GEOTECHNICAL DISCUSSION

5.1. Site Classification to AS2870

The classification has been prepared in accordance with the guidelines set out in the “Residential Slabs and Footings” Code, AS2870 – 2011.

Undisturbed samples were obtained to determine their shrink swell index.

The results are summarised in Table 5.1

Table 5.1 – Shrink Swell Summary

Location	Depth (m)	Material Description	Shrink/Swell Index (% per ΔpF)
BH1	1.4-1.65	Orange brown, light grey and dark grey silty clay	3.1
BH1	0.4-0.85	Red brown and yellow silty clay	2.6
BH4	0.5-0.7	Brown and grey silty clay	0.8
BH5	1.0-1.3	Grey brown and yellow silty gravelly clay	0.1
BH7	0.5-0.7	Grey brown and yellow silty gravelly clay	1.2
BH8	0.5-0.68	Brown and yellow silty sandy clay	1.5
BH8	1.4-1.7	Blue grey silty gravelly clay	3.6

Because of the trees and dwellings present, abnormal moisture conditions (AMC) prevail at the site. (Refer to Section 1.3.3 of AS2870).

Because of the AMC and fill present, the site is classified *a problem site (P)*. However, provided the recommendations given below are adopted and the footings bear in the underlying natural soils, the site may be reclassified *highly reactive (H1)*.

5.2. Foundation Design

The existing fill materials should not be relied upon for foundation support unless written confirmation is obtained confirming that fill was placed in a controlled manner.

Piles founded in stiff and very stiff natural materials may be proportioned using an allowable end bearing pressure of 225 kPa and 450 kPa, respectively, provided their depth to diameter ratio exceeds a value of 4. An allowable adhesion value of 20 kPa may be adopted for the portion of the shaft within the natural materials.

In order to ensure the bearing values given can be achieved, care should be taken to ensure that the base of excavations is free of all loose material prior to concreting. It is recommended that all footing excavations be protected with a layer of blinding concrete as soon as possible, preferably immediately after excavating, cleaning, inspection and approval. The possible presence of groundwater needs to be considered when drilling piers and pouring concrete.

5.3. Soil Aggressiveness

The aggressiveness or erosion potential of an environment in building materials, particularly concrete and steel is dependent on the levels of soil pH and the types of salts present, generally sulphates and chlorides. In order to determine the degree of aggressiveness, the test values obtained are compared to Tables 6.4.2 (C) and 6.5.2 (C) in AS2159 – 2009 Piling – Design and Installation. The test results are summarised in Table 5.2 below.

Table 5.2– Soil Aggressiveness Summary Table

Sample No.	Location	Depth (m)	pH	Sulfate (mg/kg)	Chloride (mg/kg)	Electrical Conductivity (dS/m)	
						EC _{1:5}	EC _e
S1	BH1	0.4	5.1	180	190	0.295	2.7
S2	BH3	0.4	5.5	90	60	0.145	1.3
S3	BH4	0.4	5.6	360	2400	0.388	3.5
S4	BH5	0.4	5.7	30	40	0.082	0.7
S5	BH7	0.3	5.8	40	140	0.162	1.5
S6	BH8	0.4	7.3	<10	20	0.054	0.5
S7	BH9	0.3	6.3	10	20	0.051	0.5

The soils on the site are cohesive in nature. Therefore, the soil conditions B are considered appropriate.

A review of the durability aspects indicates that:

- pH : minimum value of 5.1
- SO₄ : maximum value of 360 mg/kg (ppm) > 5000 ppm
- Cl : maximum value of 240 mg/kg (ppm) < 5000 ppm
- EC_e : maximum value of 3.5 dS/m

In accordance with AS2159-2009, the exposure classification for the onsite soils is non-aggressive to steel and mildly aggressive to concrete. In accordance with AS2870-2011, the soils are classified as A2.

Reference to DLWC (2002) "Site Investigations for Urban Salinity" indicates that the EC_e value of 0.3 and 0.4dS/m are consistent with the presence of non-saline and slightly saline soils.

6. ACID SULFATE SOIL ASSESSMENT

6.1. Introduction

ASS is the common name given to sediments and soils containing iron sulfides which, when exposed to oxygen generate sulfuric acid. Natural processes formed many acid sulfate sediments when certain conditions existed in the Holocene geological period (the last 10,000 years). Formation conditions require the presence of iron-rich sediments, sulfate (usually from seawater), removal of reaction products such as bicarbonate, the presence of sulfate reducing bacteria and a plentiful supply of organic matter. It should be noted that these conditions exist in mangroves, salt marsh vegetation or tidal areas, and at the bottom of coastal rivers and lakes.

The relatively specific conditions under which acid sulfate soils are formed usually limit their occurrence to low lying parts of coastal floodplains, rivers and creeks. This includes areas with saline or brackish water such as deltas, coastal flats, backswamps and seasonal or permanent freshwater swamps that were formerly brackish. Due to flooding and stormwater erosion, these sulfidic sediments may continue to be re-distributed through the sands and sediments of the estuarine floodplain region. Sulfidic sediment may be found at any depth in suitable coastal sediments – usually beneath the water table.

Any lowering in the water table that covers and protects potential ASS will result in their aeration and the exposure of iron sulfide sediments to oxygen. The lowering in the water table can occur naturally due to seasonal fluctuations and drought or any human intervention, when carrying out any excavations during site development. Potential ASS can also be exposed to air during physical disturbance with the material at the disturbance face, as well as the extracted material, both potentially being oxidised. The oxidation of iron sulfide sediments in potential ASS results in ASS soils.

Successful management of areas with ASS is possible but must take into account the specific nature of the site and the environmental consequences of development. While it is preferable that sites exhibiting acid sulfate characteristics are not disturbed, management techniques have been devised to minimise and manage impacts in certain circumstances.

When works involving the disturbance of soil or the change of groundwater levels are proposed in coastal areas, a preliminary assessment should be undertaken to determine whether acid sulfate soils are present and if the proposed works are likely to disturb these soils.

6.2. Presence of ASS

Reference to the Wallsend ASS Risk Map indicates the property is within an area there is a high probability that ASS will be encountered. The map shows the area is an estuarine backswamp. It should be noted that maps are a guide only.

The following geomorphic or site criteria are normally used to determine if acid sulfate soils are likely to be present:

- sediments of recent geological age (Holocene)
- soil horizons less than 5 in AHD
- marine or estuarine sediments and tidal lakes
- in coastal wetlands or back swamp areas

6.3. Assessment

In order to assess the significance of the ASS potential, the laboratory results carried out were compared to action criteria contained in ASSM (1998) summarised in Table 6.1. The action criteria trigger the need to prepare an ASSMP and are based on the percentage of oxidisable sulphur (or equivalent TPA and TSA) for broad categories of soil types. Works in soils that exceed these action criteria must prepare a management plan and obtain development consent.

As the soils encountered on the site primarily consisted of clays, the fine texture grade criteria are the most appropriate and have been adopted for this assessment.

Table 6.1 – ASS Action Criteria

Type of material		Action Criteria if 1-1000 tonnes ASS disturbed		Action Criteria if more than 1000 tonnes ASS disturbed	
Texture Range (McDonald et al 1990)	Approx. clay content (%<0.02mm)	Sulphur Trail %S oxidisable (oven dry basis) eg S_{TOS} or S_{POS}	Acid Trail Mol H^+ /tonne (oven dry basis) eg TPA or TSA_s	Sulphur Trail %S oxidisable (oven dry basis) eg S_{TOS} or S_{POS}	Acid Trail Mol H^+ /tonne (oven dry basis) eg TPA or TSA_s
Coarse Texture (CT) Sands to loamy sands	≥ 5	0.03	18	0.03	18
Medium Texture (MT) Sandy loams to light clays	5-50	0.06	36	0.03	18
Fine Texture (FT) Medium to heavy clays and silty clays	≥ 40	0.1	62	0.03	18

The laboratory test results are summarised in relation to the action criteria in Table 6.2 and 6.2A.

Table 6.2 – SPOCAS TEST RESULTS SUMMARY

Analysis	Unit	LOR	ASS1 BH1 @ 0.5m	ASS2 BH1 @ 1.0m	ASS3 BH1 @ 1.5m	ASS4 BH1 @ 2.0m	Action Criteria ¹ <1000 tonnes disturbed
pH before Oxidation	NA	0.1	4.2	5.8	4.6	4.5	-
pH after Oxidation	NA	0.1	4.4	7.0	4.1	4.5	<3 (high risk)
S (POS)	%	0.02	0.052	0.035	0.031	0.048	0.1
TPA	mole/tonne	2	101	<2	100	136	62
TSA	mole/tonne	2	43	<2	75	88	62

¹ = ASSMAC (1998)

Action Criteria Exceeded

Table 6.2A – SPOCAS TEST RESULTS SUMMARY

Analysis	Unit	LOR	ASS5 BH1 @ 3.0m	ASS6 BH5 @ 0.5m	ASS7 BH5 @ 1.0m	ASS8 BH8 @ 2.5m	Action Criteria ¹ <1000 tonnes disturbed
pH before Oxidation	NA	0.1	4.0	4.8	4.7	5.2	-
pH after Oxidation	NA	0.1	4.1	4.6	4.3	4.9	<3 (high risk)
S (POS)	%	0.02	<0.02	<0.02	<0.002	0.051	0.1
TPA	mole/tonne	2	180	11	91	42	62
TSA	mole/tonne	2	88	<2	68	31	62

¹ = ASSMAC (1998)

 Action Criteria Exceeded

The results of the soil sample analyses are compared to the above criteria in Table 6.1, and the analytical laboratory reports for the testing performed are provided in Appendix B.

Some of the measured test results exceed the action criteria. Therefore, an ASS Management Plan will be required.

7. FINAL COMMENTS

During construction, should the subsurface conditions vary from those inferred above, we would be contacted to determine if any changes should be made to our recommendations.

The exposed bearing surfaces for footings should be inspected by a geotechnical engineer to ensure the allowable pressure given has been achieved.



Laurie Ihnativ
Senior Geotechnical Engineer
STS Geotechnics Pty Limited



STS Geotechnics Pty. Ltd.

Scale: Unknown

Date: October 2020

Client: NSW LAND & HOUSING CORPORATION C/- SMEC AUSTRALIA

**GEOTECHNICAL INVESTIGATION—38-44 JOHN T BELL DRIVE AND
31-35 MATFEN CLOSE, MARYLAND
BOREHOLE AND PENETROMETER LOCATIONS**

Project No.
30615/4133D-G

Drawing No: 20/3519

NOTES RELATING TO GEOTECHNICAL REPORTS

Introduction

These notes have been provided to outline the methodology and limitations inherent in geotechnical reporting. The issues discussed are not relevant to all reports and further advice should be sought if there are any queries regarding any advice or report.

When copies of reports are made, they should be reproduced in full.

Geotechnical Reports

Geotechnical reports are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work and is supplemented by knowledge of the local geology and experience of the range of properties that may be exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report has been prepared for a specific purpose (eg. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (eg. a twenty storey building). In such cases, the report and the sufficiency of the existing work should be reviewed by STS Geotechnics Pty Limited in the light of the new proposal.

Every care is taken with the report content, however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities.
- The actions of contractors responding to commercial pressures.

If these occur, STS Geotechnics Pty Limited would be pleased to resolve the matter through further investigation, analysis or advice.

Unforeseen Conditions

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report, STS

Geotechnics Pty Limited should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows re-interpretation and assessment of the implications for future work.

Subsurface Information

Logs of a borehole, recovered core, test pit, excavated face or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling and/or observation spacings and the ground conditions. It is not always possible or economic to obtain continuous high quality data. It should also be recognised that the volume or material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long term monitoring of groundwater levels may be required to adequately identify groundwater conditions.

Supply of Geotechnical Information or Tendering Purposes

It is recommended tenderers are provided with as much geological and geotechnical information that is available and that where there are uncertainties regarding the ground conditions, prospective tenders should be provided with comments discussing the range of likely conditions in addition to the investigation data.

APPENDIX A – BOREHOLE LOGS AND EXPLANATION SHEETS

Client: NSW Land & Housing Corporation C/- SMEC Australia		Project / STS No. 30615/4133D-G		BOREHOLE NO.: BH 1		
Project: 38-44 John T Bell Drive and 31-35 Matfen Close, Maryland		Date: October 1, 2020				
Location: Refer to Drawing No. 20/3519		Logged: JK Checked By: SS		Sheet 1 of 1		
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			FILL: SILTY CLAY: light brown, medium plasticity	CL	SOFT TO FIRM	M
	S1 @ 0.4 m					
	ASS1 @ 0.5 m	0.5	FILL: SILTY CLAY: mottled orange brown, light grey and dark grey, medium to high plasticity	CL/CH	GENERALLY STIFF	M
	U50 0.6-0.85 m					
	ASS2 @ 1.0 m	1.0				
	U50 1.4-1.65 m		SILTY CLAY: dark grey with occasional orange brown, medium to high plasticity	CL/CH	FIRM TO STIFF	M
	ASS3 @ 1.5 m	1.5				
	ASS4 @ 2.0 m	2.0			VERY STIFF	
		2.5				
			SILTY CLAY: light grey with orange brown, high plasticity	CH	VERY STIFF	M
	ASS5 @ 3.0 m		BOREHOLE DISCONTINUED AT 3.0 M ON SILTY CLAY			
D - disturbed sample U - undisturbed tube sample B - bulk sample				Contractor: STS		
WT - level of water table or free water N - Standard Penetration Test (SPT)				Equipment: Mini Christie		
S - jar sample				Hole Diameter (mm): 100		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols				Angle from Vertical (°): 0		
				Drill Bit: Spiral		

Client: NSW Land & Housing Corporation C/- SMEC Australia		Project / STS No. 30615/4133D-G		BOREHOLE NO.: BH 2		
Project: 38-44 John T Bell Drive and 31-35 Matfen Close, Maryland		Date: October 1, 2020				
Location: Refer to Drawing No. 20/3519		Logged: JK Checked By: SS		Sheet 1 of 1		
W A T E R L E V E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			FILL: SILTY CLAY: light brown, low plasticity	CL	SOFT	M
		0.5	FILL: SILTY CLAY: mottled orange brown, light grey, dark grey and yellow brown, medium to high plasticity, trace of gravel	CL/CH	SOFT TO FIRM ----- GENERALLY FIRM TO STIFF	M
		1.0				
		1.5				
		2.0	SILTY CLAY: light grey with dark grey and orange brown, high plasticity, occasional organic matter	CH	FIRM TO STIFF ----- STIFF	M
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M ON SILTY CLAY			
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Mini Christie Hole Diameter (mm): 100 Angle from Vertical (°): 0 Drill Bit: Spiral		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols						

Form: I1-2

Client: NSW Land & Housing Corporation C/- SMEC Australia		Project / STS No. 30615/4133D-G		BOREHOLE NO.: BH 4		
Project: 38-44 John T Bell Drive and 31-35 Matfen Close, Maryland		Date: October 1, 2020				
Location: Refer to Drawing No. 20/3519		Logged: JK Checked By: SS		Sheet 1 of 1		
W A T E R L E V E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S3 @ 0.3 m		FILL: SILTY SANDY CLAY: light brown, fine grained, low plasticity	CL	FIRM TO STIFF	D-M
			FILL: SILTY CLAY: mottled orange brown, light grey, yellow brown and dark grey, medium to high plasticity	CL/CH	GENERALLY FIRM TO STIFF	D-M
	U50	0.5				
		1.0				
		1.5	SILTY CLAY: dark grey, high plasticity	CH	STIFF	M
		2.0			----- VERY STIFF	
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M ON SILTY CLAY			
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Mini Christie Hole Diameter (mm): 100 Angle from Vertical (°): 0 Drill Bit: Spiral		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols						

Client: NSW Land & Housing Corporation C/- SMEC Australia		Project / STS No. 30615/4133D-G		BOREHOLE NO.: BH 5		
Project: 38-44 John T Bell Drive and 31-35 Matfen Close, Maryland		Date: October 1, 2020				
Location: Refer to Drawing No. 20/3519		Logged: JK Checked By: SS		Sheet 1 of 1		
W A T T A B E R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			FILL: SILTY CLAY: dark brown, medium plasticity, trace of fine sand	CL	SOFT TO FIRM	M
	S4 @ 0.4 m					
	ASS6 @ 0.5 m	0.5	FILL: SILTY CLAY: mottled light grey with orange brown and yellow brown and dark grey, medium to high plasticity, trace of fine sand	CL/CH	GENERALLY FIRM TO STIFF AND STIFF	M
	ASS7 @ 1.0 m					
	U50 1.0-1.3 m	1.0				
						M-W
		1.5	SILTY SANDY CLAY: dark grey, fine grained, medium plasticity	CL	SOFT TO FIRM	M-W
		2.0				
	ASS8 @ 2.5 m	2.5	SILTY CLAY: orange brown with light grey, medium to high plasticity	CL/CH	FIRM TO STIFF ----- VERY STIFF	M
			BOREHOLE DISCONTINUED AT 3.0 M ON SILTY CLAY			
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Mini Christie Hole Diameter (mm): 100 Angle from Vertical (°): 0 Drill Bit: Spiral		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols						

Client: NSW Land & Housing Corporation C/- SMEC Australia		Project / STS No. 30615/4133D-G		BOREHOLE NO.: BH 6		
Project: 38-44 John T Bell Drive and 31-35 Matfen Close, Maryland		Date: October 1, 2020				
Location: Refer to Drawing No. 20/3519		Logged: JK Checked By: SS		Sheet 1 of 1		
W A T E R L E V E L	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			FILL: SILTY CLAY: dark brown, medium plasticity	CL	SOFT TO FIRM	M
			FILL: SILTY CLAY: mottled orange brown, light grey, yellow brown and dark grey, medium to high plasticity	CL/CH	GENERALLY VERY STIFF	M
		0.5				
		1.0				
			SILTY SANDY CLAY: light grey with orange brown, fine grained, medium to high plasticity	CL/CH	STIFF	M
		1.5				
					VERY STIFF	
		2.0				
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M ON SILTY SANDY CLAY			M-W
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Mini Christie Hole Diameter (mm): 100		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols				Angle from Vertical (°): 0 Drill Bit: Spiral		

Client: NSW Land & Housing Corporation C/- SMEC Australia		Project / STS No. 30615/4133D-G		BOREHOLE NO.: BH 7		
Project: 38-44 John T Bell Drive and 31-35 Matfen Close, Maryland		Date: October 1, 2020				
Location: Refer to Drawing No. 20/3519		Logged: JK Checked By: SS		Sheet 1 of 1		
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			TOPSOIL: SILTY CLAY: dark brown, medium plasticity	CL	SOFT TO FIRM	M
	SS @ 0.3 m		FILL: SILTY CLAY: mottled light grey, light brown, orange brown and yellow brown, medium to high plasticity, trace of gravel	CL/CH	GENERALLY FIRM TO STIFF	M
	U50 0.5-0.7 m	0.5				
		1.0				
		1.5				
		2.0	SILTY CLAY: dark grey, medium to high plasticity	CL/CH	STIFF TO VERY STIFF	M
		2.5	SILTY CLAY: orange brown with light grey, medium to high plasticity	CL/CH	VERY STIFF	M
			BOREHOLE DISCONTINUED AT 3.0 M ON SILTY CLAY			
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Mini Christie Hole Diameter (mm): 100 Angle from Vertical (°): 0 Drill Bit: Spiral		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols						

[illegible]

Revision: 1

Dynamic Cone Penetrometer Test Report

Project: 38-44 JOHN T BELL DRIVE & 31-35 MATFEN CLOSE, MARYLAND

Project No.: 30615/4133D

Client: NSW LAND & HOUSING CORPORATION C/- SMEC AUSTRALIA

Report No.: 20/3519

Address: 20 Berry Street, North Sydney

Report Date: 12/10/2020

Test Method: AS 1289.6.3.2

Page: 1 of 2



Accredited for compliance with ISO/IEC
17025 - Testing
The results of the tests, calibrations and/or
measurements included in this document are
traceable to Australian/national standards
NATA Accreditation Number 2750

Site No.	P1	P2	P3	P4	P5	P6
Location	Refer to Drawing No. 20/3519	Refer to Drawing No. 20/3519	Refer to Drawing No. 20/3519	Refer to Drawing No. 20/3519	Refer to Drawing No. 20/3519	Refer to Drawing No. 20/3519
Date Tested	1/10/2020	1/10/2020	1/10/2020	1/10/2020	1/10/2020	1/10/2020
Starting Level	Surface Level	Surface Level	Surface Level	Surface Level	Surface Level	Surface Level
Depth (m)	Penetration Resistance (blows / 150mm)					
0.00 - 0.15	1	1	1	1	1	1
0.15 - 0.30	2	2	2	3	3	2
0.30 - 0.45	4	2	4	4	4	3
0.45 - 0.60	2	4	4	2	3	3
0.60 - 0.75	3	5	5	3	5	3
0.75 - 0.90	5	5	6	3	5	4
0.90 - 1.05	5	4	7	3	6	3
1.05 - 1.20	7	4	7	3	4	4
1.20 - 1.35	7	6	6	3	4	4
1.35 - 1.50	5	6	6	5	3	5
1.50 - 1.65	5	7	3	5	2	6
1.65 - 1.80	4	6	3	7	2	9
1.80 - 1.95	7	4	4	10	2	9
1.95 - 2.10	11	4	4	12	2	16
2.10 - 2.25	13	3	5	14	2	16
2.25 - 2.40	15	4	6	15	4	18
2.40 - 2.55	17	5	6	15	8	22
2.55 - 2.70	19	5	4	16	10	Refusal
2.70 - 2.85	22	6	5	16	10	
2.85 - 3.00	Refusal	8	5	17	11	
3.00 - 3.15		Discontinued	Discontinued	Discontinued	Discontinued	
3.15 - 3.30						
3.30 - 3.45						
3.45 - 3.60						
3.60 - 3.75						

Remarks: * Pre drilled prior to testing



Approved Signatory.....

Technician: JK

Orlando Mendoza - Laboratory Manager

Dynamic Cone Penetrometer Test Report

Project: 38-44 JOHN T BELL DRIVE & 31-35 MATFEN CLOSE, MARYLAND

Project No.: 30615/4133D

Client: NSW LAND & HOUSING CORPORATION C/- SMEC AUSTRALIA

Report No.: 20/3519

Address: 20 Berry Street, North Sydney

Report Date: 12/10/2020

Test Method: AS 1289.6.3.2

Page: 2 of 2



Accredited for compliance with ISO/IEC
17025 - Testing
The results of the tests, calibrations and/or
measurements included in this document are
traceable to Australian/national standards
NATA Accreditation Number 2750

Site No.	P7	P8	P9			
Location	Refer to Drawing No. 20/3519	Refer to Drawing No. 20/3519	Refer to Drawing No. 20/3519			
Date Tested	1/10/2020	1/10/2020	1/10/2020			
Starting Level	Surface Level	Surface Level	Surface Level			
Depth (m)	Penetration Resistance (blows / 150mm)					
0.00 - 0.15	1	1	1			
0.15 - 0.30	2	2	2			
0.30 - 0.45	3	4	4			
0.45 - 0.60	3	3	3			
0.60 - 0.75	4	4	3			
0.75 - 0.90	5	6	4			
0.90 - 1.05	4	6	5			
1.05 - 1.20	3	7	6			
1.20 - 1.35	3	6	6			
1.35 - 1.50	3	5	5			
1.50 - 1.65	4	4	4			
1.65 - 1.80	6	4	4			
1.80 - 1.95	8	4	4			
1.95 - 2.10	8	6	5			
2.10 - 2.25	10	6	8			
2.25 - 2.40	12	7	9			
2.40 - 2.55	15	9	10			
2.55 - 2.70	22	10	11			
2.70 - 2.85	Refusal	10	12			
2.85 - 3.00		10	13			
3.00 - 3.15		Discontinued	Discontinued			
3.15 - 3.30						
3.30 - 3.45						
3.45 - 3.60						
3.60 - 3.75						

Remarks: * Pre drilled prior to testing



Approved Signatory.....

Technician: JK

Orlando Mendoza - Laboratory Manager

E1. CLASSIFICATION OF SOILS

E1.1 Soil Classification and the Unified System

An assessment of the site conditions usually includes an appraisal of the data available by combining values of engineering properties obtained by the site investigation with descriptions, from visual observation of the materials present on site.

The system used by STS Geotechnics Pty Ltd (STS) in the identification of soil is the Unified Soil Classification system (USC) which was developed by the US Army Corps of Engineers during World War II and has since gained international acceptance and has been adopted in its metricated form by the Standards Association of Australia.

The Australian Site Investigation Code (AS1726-1981, Appendix D) recommends that the description of a soil includes the USC group symbols which are an integral component of the system.

The soil description should contain the following information in order:

Soil composition

- SOIL NAME and USC classification symbol (IN BLOCK LETTERS)
- plasticity or particle characteristics
- colour
- secondary and minor constituents (name estimated proportion, plasticity or particle characteristics, colour)

Soil condition

- moisture condition
- consistency or density index

Soil structure

- structure (zoning, defects, cementing)

Soil origin

interpretation based on observation eg FILL, TOPSOIL, RESIDUAL, ALLUVIUM.

E1.2 Soil Composition

- (a) Soil Name and Classification Symbol

The USC system is summarised in Figure E1.2.1. The primary division separates soil types on the basis of particle size into:

- Coarse grained soils - more than 50% of the material less than 60 mm is larger than 0.06 mm (60 μ m).
- Fine grained soils - more than 50% of the material less than 60 mm is smaller than 0.06 mm (60 μ m).

Initial classification is by particle size as shown in Table E1.2.1. Further classification of fine grained soils is based on plasticity.

TABLE E1.2.1 - CLASSIFICATION BY PARTICLE SIZE

NAME	SUB-DIVISION	SIZE
Clay (1)		< 2 μ m
Silt (2)		2 μ m to 60 μ m
Sand	Fine Medium Coarse	60 μ m to 200 μ m 200 μ m to 600 μ m 600 μ m to 2 mm
Gravel (3)	Fine Medium Coarse	2 mm to 6 mm 6 mm to 20 mm 20 mm to 60 mm
Cobbles (3)		60 mm to 200 mm
Boulders (3)		> 200 mm

Where a soil contains an appropriate amount of secondary material, the name includes each of the secondary components (greater than 12%) in increasing order of significance, eg sandy silty clay.

Minor components of a soil are included in the description by means of the terms "some" and "trace" as defined in Table E1.2.2.

TABLE E1.2.2 - MINOR SOIL COMPONENTS

TERM	DESCRIPTION	APPROXIMATE PROPORTION (%)
Trace	presence just detectable, little or no influence on soil properties	0-5
Some	presence easily detectable, little influence on soil properties	5-12

The USC group symbols should be included with each soil description as shown in Table E1.2.3

TABLE E1.2.3 - SOIL GROUP SYMBOLS

SOIL TYPE	PREFIX
Gravel	G
Sand	S
Silt	M
Clay	C
Organic	O
Peat	Pt

The group symbols are combined with qualifiers which indicate grading, plasticity or secondary components as shown on Table E1.2.4

TABLE E1.2.4 - SOIL GROUP QUALIFIERS

SUBGROUP	SUFFIX
Well graded	W
Poorly Graded	P
Silty	M
Clayey	C
Liquid Limit <50% - low to medium plasticity	L
Liquid Limit >50% - medium to high plasticity	H

(b) Grading

“Well graded”	Good representation of all particle sizes from the largest to the smallest.
“Poorly graded”	One or more intermediate sizes poorly represented
“Gap graded”	One or more intermediate sizes absent
“Uniformly graded”	Essentially single size material.

(c) Particle shape and texture

The shape and surface texture of the coarse grained particles should be described.

Angularity may be expressed as “rounded”, “sub-rounded”, “sub-angular” or “angular”.

Particle **form** can be “equidimensional”, “flat” or “elongate”.

Surface texture can be “glassy”, “smooth”, “rough”, “pitted” or “striated”.

(d) Colour

The colour of the soil should be described in the moist condition using simple terms such as:

Black	White	Grey	Red
Brown	Orange	Yellow	Green
Blue			

These may be modified as necessary by “light” or “dark”. Borderline colours may be described as a combination of two colours, eg red-brown.

For soils that contain more than one colour terms such as:

- Speckled Very small (<10 mm dia) patches
- Mottled Irregular
- Blotched Large irregular (>75 mm dia)
- Streaked Randomly oriented streaks

(e) Minor Components

Secondary and minor components should be individually described in a similar manner to the dominant component.

E1.3 Soil Condition

(a) Moisture

Soil moisture condition is described as “dry”, “moist” or “wet”.

The moisture categories are defined as:

Dry (D) - Little or no moisture evident. Soils are running. Moist (M) - Darkened in colour with cool feel. Granular soil particles tend to adhere. No free water evident upon remoulding of cohesive soils.

In addition the moisture content of cohesive soils can be estimated in relation to their liquid or plastic limit.

(b) Consistency

Estimates of the consistency of a clay or silt soil may be made from manual examination, hand penetrometer test, SPT results or from laboratory tests to determine undrained shear or unconfined compressive strengths. The classification of consistency is defined in Table E1.3.1.

TABLE E1.3.1 - CONSISTENCY OF FINE-GRAINED SOILS

TERM	UNCONFINED STRENGTH (kPa)	FIELD IDENTIFICATION
Very Soft	<25	Easily penetrated by fist. Sample exudes between fingers when squeezed in the fist.
Soft	25 - 50	Easily moulded in fingers. Easily penetrated 50 mm by thumb.
Firm	50 - 100	Can be moulded by strong pressure in the fingers. Penetrated only with great effort.
Stiff	100 - 200	Cannot be moulded in fingers. Indented by thumb but penetrated only with great effort.
Very Stiff	200 - 400	Very tough. Difficult to cut with knife. Readily indented with thumb nail.
Hard	>400	Brittle, can just be scratched with thumb nail. Tends to break into fragments.

Unconfined compressive strength as derived by a hand penetrometer can be taken as approximately double the undrained shear strength ($q_u = 2 c_u$).

(c) Density Index

The insitu density index of granular soils can be assessed from the results of SPT or cone penetrometer tests. Density index should not be estimated visually.

TABLE E1.3.2 - DENSITY OF GRANULAR SOILS

TERM	SPT N VALUE	STATIC CONE VALUE q _c (MPa)	DENSITY INDEX (%)
Very Loose	0 - 3	0 - 2	0 - 15
Loose	3 - 8	2 - 5	15 - 35
Medium Dense	8 - 25	5 - 15	35 - 65
Dense	25 - 42	15 - 20	65 - 85
Very Dense	>42	>20	>85

E1.4 Soil Structure

(a) Zoning

A sample may consist of several zones differing in colour, grain size or other properties. Terms to classify these zones are:

Layer - continuous across exposure or sample

Lens - discontinuous with lenticular shape

Pocket - irregular inclusion

Each zone should be described, their distinguishing features, and the nature of the interzone boundaries.

(b) Defects

Defects which are present in the sample can include:

- fissures
- roots (containing organic matter)
- tubes (hollow)
- casts (infilled)

Defects should be described giving details of dimensions and frequency. Fissure orientation, planarity, surface condition and infilling should be noted. If there is a tendency to break into blocks, block dimensions should be recorded

E1.5 Soil Origin

Information which may be interpretative but which may contribute to the usefulness of the material description should be included. The most common interpreted feature is the origin of the soil. The assessment of the probable origin is based on the soil material description, soil structure and its relationship to other soil and rock materials.

Common terms used are:

“Residual Soil” - Material which appears to have been derived by weathering from the underlying rock. There is no evidence of transport.

“Colluvium” - Material which appears to have been transported from its original location. The method of movement is usually the combination of gravity and erosion.

“Landslide Debris” - An extreme form of colluvium where the soil has been transported by mass movement. The material is obviously distributed and contains distinct defects related to the slope failure.

“Alluvium” - Material which has been transported essentially by water. usually associated with former stream activity.

“Fill” - Material which has been transported and placed by man. This can range from natural soils which have been

placed in a controlled manner in engineering construction to dumped waste material. A description of the constituents should include an assessment of the method of placement.

E1.6 Fine Grained Soils

The physical properties of fine grained soils are dominated by silts and clays.

The definition of clay and silt soils is governed by their Atterberg Limits. Clay soils are characterised by the properties of cohesion and plasticity with cohesion defines as the ability to deform without rupture. Silts exhibit cohesion but have low plasticity or are non-plastic.

The field characteristics of clay soils include:

- dry lumps have appreciable dry strength and cannot be powdered
- volume changes occur with moisture content variation
- feels smooth when moist with a greasy appearance when cut.

The field characteristics of silt soils include:

- dry lumps have negligible dry strength and can be powdered easily
- dilatancy - an increase in volume due to shearing - is indicated by the presence of a shiny film of water after a hand sample is shaken. The water disappears upon remoulding. Very fine grained sands may also exhibit dilatancy.
- low plasticity index
- feels gritty to the teeth

E1.7 Organic Soils

Organic soils are distinguished from other soils by their appreciable content of vegetable matter, usually derived from plant remains.

The soil usually has a distinctive smell and low bulk density.

The USC system uses the symbol Pt for partly decomposed organic material. The O symbol is combined with suffixes “O” or “H” depending on plasticity.

Where roots or root fibres are present their frequency and the depth to which they are encountered should be recorded. The presence of roots or root fibres does not necessarily mean the material is an “organic material” by classification.

Coal and lignite should be described as such and not simply as organic matter.

APPENDIX B – LABORATORY TEST RESULTS

STS Geotechnics Pty Ltd

14/1 Cowpasture Place, Wetherill Park NSW 2164

Phone: (02)9756 2166 | Email: enquiries@stsgo.com.au

**Shrink Swell Index Report**

Project: 38-44 JOHN T BELL DRIVE & 31-35 MATFEN CLOSE, MARYLAND

Client: **NSW LAHC C/- SMEC AUSTRALIA**

Address: PO Box 1052, North Sydney 2059

Test Method: AS1289.7.1.1

Project No.: 30615

Report No.: 20/3452

Report Date: 7/10/2020

Page: 1 of 2

Sampling Procedure: AS 1289.1.3.1 Clause 3.1.3.2 - Thin Walled Sampler

STS / Sample No.		4133D-L/1	4133D-L/2	4133D-L/3	4133D-L/4	4133D-L/5	4133D-L/6
Sample Location		Borehole 1 Refer to Drawing	Borehole 4 Refer to Drawing	Borehole 5 Refer to Drawing	Borehole 7 Refer to Drawing	Borehole 8 Refer to Drawing	Borehole 8 Refer to Drawing
Material Description		Silty Clay, orange/grey/ brown, trace of gravel	Silty Gravelly Clay, yellow brown/grey	Silty Gravelly Clay, grey brown/yellow	Silty Gravelly Clay, grey brown/yellow	Silty Sandy Clay, brown/yellow, trace of gravel	Silty Gravelly Clay, blue/grey
Depth (m)		1.4 - 1.65	0.5 - 0.7	1.0 - 1.3	0.5 - 0.7	0.5 - 0.68	1.4 - 1.7
Sample Date		1/10/2020	1/10/2020	1/10/2020	1/10/2020	1/10/2020	1/10/2020
Shrink	Moisture Content (%)	26.1	16.2	21.3	19.9	25.8	31.8
	Soil Crumbling	Nil	Nil	Nil	Nil	Nil	Nil
	Extent of Cracking	Fine Cracks	Fine Cracks	Open Cracks	Open Cracks	Fine Cracks	Open Cracks
	Strain (%)	4.3	1.1	0.1	2.1	2.8	6.4
Swell	Moisture Content Initial (%)	26.4	20.3	22.4	20.1	27.1	27.9
	Moisture Content Final (%)	32.5	24.7	29.4	24.0	29.3	33.7
	Strain (%)	2.5	0.7	0.0	0.2	0.0	0.0
Inert Inclusions (%)		<5	<20	<20	<25	<5	<25
Shrink Swell Index (%)		3.1	0.8	0.1	1.2	1.5	3.6

Remarks:

Accredited for compliance with ISO/IEC
17025 - TestingThe results of the tests, calibrations and/or
measurements included in this document are
traceable to Australian/national standards
NATA Accreditation Number 2750

A handwritten signature in black ink, appearing to read 'Orlando Mendoza'.

Approved Signatory.....

Orlando Mendoza - Laboratory Manager

Technician: DH

STS Geotechnics Pty Ltd

14/1 Cowpasture Place, Wetherill Park NSW 2164

Phone: (02)9756 2166 | Email: enquiries@stsgео.com.au

**Shrink Swell Index Report**

Project: 38-44 JOHN T BELL DRIVE & 31-35 MATFEN CLOSE, MARYLAND

Client: **NSW LAHC C/- SMEC AUSTRALIA**

Address: PO Box 1052, North Sydney 2059

Test Method: AS1289.7.1.1

Project No.: 30615

Report No.: 20/3452

Report Date: 7/10/2020

Page: 2 of 2

Sampling Procedure: AS 1289.1.3.1 Clause 3.1.3.2 - Thin Walled Sampler

STS / Sample No.		4133D-L/7					
Sample Location		Borehole 1 Refer to Drawing					
Material Description		Silty Clay, red brown/yellow, trace of gravel					
Depth (m)		0.6 - 0.85					
Sample Date		1/10/2020					
Shrink	Moisture Content (%)	23.5					
	Soil Crumbling	Nil					
	Extent of Cracking	Open Cracks					
	Strain (%)	4.2					
Swell	Moisture Content Initial (%)	24.6					
	Moisture Content Final (%)	29.5					
	Strain (%)	0.9					
Inert Inclusions (%)		<5					
Shrink Swell Index (%)		2.6					

Remarks:

Accredited for compliance with ISO/IEC
17025 - TestingThe results of the tests, calibrations and/or
measurements included in this document are
traceable to Australian/national standards
NATA Accreditation Number 2750

A handwritten signature in black ink, appearing to read 'Orlando Mendoza'.

Approved Signatory.....

Technician: DH

Orlando Mendoza - Laboratory Manager

CERTIFICATE OF ANALYSIS

Work Order : **ES2034713**
Client : **STS Geotechnics**
Contact : **ENQUIRES STS**
Address : Unit 14/1 Cowpasture Place
 Wetherill Park 2164
Telephone : ----
Project : 30055/30060/30614/30615/30616
Order number : E-2020-0389
C-O-C number : ----
Sampler : MB/JK
Site : ----
Quote number : EN/222
No. of samples received : 33
No. of samples analysed : 33

Page : 1 of 13
Laboratory : Environmental Division Sydney
Contact : Customer Services ES
Address : 277-289 Woodpark Road Smithfield NSW Australia 2164
Telephone : +61-2-8784 8555
Date Samples Received : 02-Oct-2020 14:00
Date Analysis Commenced : 06-Oct-2020
Issue Date : 12-Oct-2020 18:55



Accreditation No. 825
 Accredited for compliance with
 ISO/IEC 17025 - Testing

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

<i>Signatories</i>	<i>Position</i>	<i>Accreditation Category</i>
Ankit Joshi	Inorganic Chemist	Sydney Inorganics, Smithfield, NSW
Ben Felgendrejeris	Senior Acid Sulfate Soil Chemist	Brisbane Acid Sulphate Soils, Stafford, QLD
Celine Conceicao	Senior Spectroscopist	Sydney Inorganics, Smithfield, NSW



General Comments

The analytical procedures used by ALS have been developed from established internationally recognised procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are fully validated and are often at the client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Where a result is required to meet compliance limits the associated uncertainty must be considered. Refer to the ALS Contact for details.

Key : CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.

LOR = Limit of reporting

^ = This result is computed from individual analyte detections at or above the level of reporting

Ø = ALS is not NATA accredited for these tests.

~ = Indicates an estimated value.

- ASS: EA029 (SPOCAS): Laboratory determinations of ANC needs to be corroborated by effectiveness of the measured ANC in relation to incubation ANC. Unless corroborated, the results of ANC testing should be discounted when determining Net Acidity for comparison with action criteria, or for the determination of the acidity hazard and required liming amounts.
- ASS: EA029 (SPOCAS): Liming rate is calculated and reported on a dry weight basis assuming use of fine agricultural lime (CaCO_3) and using a safety factor of 1.5 to allow for non-homogeneous mixing and poor reactivity of lime. For conversion of Liming Rate from kg/t dry weight to kg/m³ in-situ soil, multiply reported results x wet bulk density of soil in t/m³.



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30055/7020	30055/7025	30055/7026	30060/1280	30060/1281
Client sampling date / time					01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00
Compound	CAS Number	LOR	Unit		ES2034713-001	ES2034713-002	ES2034713-003	ES2034713-004	ES2034713-005
				Result	Result	Result	Result	Result	Result
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit		5.9	7.4	7.0	7.3	7.3
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm		29	319	122	50	178
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	0.1	%		15.0	11.9	15.2	18.7	11.2
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		10	30	30	10	<10



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30060/1282	30060/1283	30060/1288	30060/1289	30614/S1
Client sampling date / time					01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	30-Sep-2020 00:00
Compound	CAS Number	LOR	Unit		ES2034713-006	ES2034713-007	ES2034713-008	ES2034713-009	ES2034713-010
				Result	Result	Result	Result	Result	Result
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit		6.2	6.1	5.4	5.9	5.6
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm		20	17	76	47	30
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	0.1	%		4.5	5.2	18.7	9.1	19.1
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		<10	<10	20	<10	<10
ED045G: Chloride by Discrete Analyser									
Chloride	16887-00-6	10	mg/kg		----	----	----	----	30



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30614/S2	30615/S1	30615/S2	30615/S3	30615/S4
Client sampling date / time					30-Sep-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00
Compound	CAS Number	LOR	Unit		ES2034713-011	ES2034713-012	ES2034713-013	ES2034713-014	ES2034713-015
					Result	Result	Result	Result	Result
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit		5.6	5.1	5.5	5.6	5.7
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm		48	295	145	388	82
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	0.1	%		25.9	19.1	17.7	16.9	19.4
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		20	180	90	360	30
ED045G: Chloride by Discrete Analyser									
Chloride	16887-00-6	10	mg/kg		20	190	60	240	40

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30615/S5	30615/S6	30615/S7	30615/ASS1	30615/ASS2
Client sampling date / time				01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00
Compound	CAS Number	LOR	Unit	ES2034713-016	ES2034713-017	ES2034713-018	ES2034713-019	ES2034713-020	
				Result	Result	Result	Result	Result	
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit	5.8	7.3	6.3	----	----	
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm	162	54	51	----	----	
EA029-A: pH Measurements									
pH KCl (23A)	----	0.1	pH Unit	----	----	----	4.2	5.8	
pH OX (23B)	----	0.1	pH Unit	----	----	----	4.3	7.0	
EA029-B: Acidity Trail									
Titratable Actual Acidity (23F)	----	2	mole H+ / t	----	----	----	58	<2	
Titratable Peroxide Acidity (23G)	----	2	mole H+ / t	----	----	----	101	<2	
Titratable Sulfidic Acidity (23H)	----	2	mole H+ / t	----	----	----	43	<2	
sulfidic - Titratable Actual Acidity (s-23F)	----	0.020	% pyrite S	----	----	----	0.093	<0.020	
sulfidic - Titratable Peroxide Acidity (s-23G)	----	0.020	% pyrite S	----	----	----	0.162	<0.020	
sulfidic - Titratable Sulfidic Acidity (s-23H)	----	0.020	% pyrite S	----	----	----	0.068	<0.020	
EA029-C: Sulfur Trail									
KCl Extractable Sulfur (23Ce)	----	0.020	% S	----	----	----	0.039	0.033	
Peroxide Sulfur (23De)	----	0.020	% S	----	----	----	0.052	0.035	
Peroxide Oxidisable Sulfur (23E)	----	0.020	% S	----	----	----	<0.020	<0.020	
acidity - Peroxide Oxidisable Sulfur (a-23E)	----	10	mole H+ / t	----	----	----	<10	<10	
EA029-D: Calcium Values									
KCl Extractable Calcium (23Vh)	----	0.020	% Ca	----	----	----	0.062	0.033	
Peroxide Calcium (23Wh)	----	0.020	% Ca	----	----	----	0.062	0.033	
Acid Reacted Calcium (23X)	----	0.020	% Ca	----	----	----	<0.020	<0.020	
acidity - Acid Reacted Calcium (a-23X)	----	10	mole H+ / t	----	----	----	<10	<10	
sulfidic - Acid Reacted Calcium (s-23X)	----	0.020	% S	----	----	----	<0.020	<0.020	
EA029-E: Magnesium Values									
KCl Extractable Magnesium (23Sm)	----	0.020	% Mg	----	----	----	0.158	0.212	
Peroxide Magnesium (23Tm)	----	0.020	% Mg	----	----	----	0.160	0.212	
Acid Reacted Magnesium (23U)	----	0.020	% Mg	----	----	----	<0.020	<0.020	
Acidity - Acid Reacted Magnesium (a-23U)	----	10	mole H+ / t	----	----	----	<10	<10	
sulfidic - Acid Reacted Magnesium (s-23U)	----	0.020	% S	----	----	----	<0.020	<0.020	
EA029-F: Excess Acid Neutralising Capacity									



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30615/S5	30615/S6	30615/S7	30615/ASS1	30615/ASS2
Client sampling date / time					01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00
Compound	CAS Number	LOR	Unit		ES2034713-016	ES2034713-017	ES2034713-018	ES2034713-019	ES2034713-020
					Result	Result	Result	Result	Result
EA029-F: Excess Acid Neutralising Capacity - Continued									
Excess Acid Neutralising Capacity (23Q)	----	0.020	% CaCO3	----	----	----	----	----	0.207
acidity - Excess Acid Neutralising Capacity (a-23Q)	----	10	mole H+ / t	----	----	----	----	----	41
sulfidic - Excess Acid Neutralising Capacity (s-23Q)	----	0.020	% S	----	----	----	----	----	0.066
EA029-G: Retained Acidity									
HCl Extractable Sulfur (20Be)	----	0.020	% S	----	----	----	----	0.046	----
Net Acid Soluble Sulfur (20Je)	----	0.020	% S	----	----	----	----	<0.020	----
acidity - Net Acid Soluble Sulfur (a-20J)	----	10	mole H+ / t	----	----	----	----	<10	----
sulfidic - Net Acid Soluble Sulfur (s-20J)	----	0.020	% pyrite S	----	----	----	----	<0.020	----
EA029-H: Acid Base Accounting									
ANC Fineness Factor	----	0.5	-	----	----	----	----	1.5	1.5
Net Acidity (sulfur units)	----	0.02	% S	----	----	----	----	0.11	<0.02
Net Acidity (acidity units)	----	10	mole H+ / t	----	----	----	----	70	<10
Liming Rate	----	1	kg CaCO3/t	----	----	----	----	5	<1
Net Acidity excluding ANC (sulfur units)	----	0.02	% S	----	----	----	----	0.11	<0.02
Net Acidity excluding ANC (acidity units)	----	10	mole H+ / t	----	----	----	----	70	<10
Liming Rate excluding ANC	----	1	kg CaCO3/t	----	----	----	----	5	<1
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	0.1	%	----	18.9	11.6	18.6	----	----
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg	----	40	<10	10	----	----
ED045G: Chloride by Discrete Analyser									
Chloride	16887-00-6	10	mg/kg	----	140	20	20	----	----



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30615/ASS3	30615/ASS4	30615/ASS5	30615/ASS6	30615/ASS7
Client sampling date / time					01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00
Compound	CAS Number	LOR	Unit		ES2034713-021	ES2034713-022	ES2034713-023	ES2034713-024	ES2034713-025
					Result	Result	Result	Result	Result
EA029-A: pH Measurements									
pH KCl (23A)	----	0.1	pH Unit		4.6	4.5	4.0	4.8	4.7
pH OX (23B)	----	0.1	pH Unit		4.1	4.5	4.1	4.6	4.3
EA029-B: Acidity Trail									
Titrateable Actual Acidity (23F)	----	2	mole H+ / t		25	48	120	10	24
Titrateable Peroxide Acidity (23G)	----	2	mole H+ / t		100	136	180	11	91
Titrateable Sulfidic Acidity (23H)	----	2	mole H+ / t		75	88	60	<2	68
sulfidic - Titrateable Actual Acidity (s-23F)	----	0.020	% pyrite S		0.040	0.077	0.192	<0.020	0.038
sulfidic - Titrateable Peroxide Acidity (s-23G)	----	0.020	% pyrite S		0.161	0.218	0.289	<0.020	0.146
sulfidic - Titrateable Sulfidic Acidity (s-23H)	----	0.020	% pyrite S		0.121	0.141	0.096	<0.020	0.108
EA029-C: Sulfur Trail									
KCl Extractable Sulfur (23Ce)	----	0.020	% S		0.032	<0.020	0.031	<0.020	0.028
Peroxide Sulfur (23De)	----	0.020	% S		0.063	0.048	0.050	<0.020	0.044
Peroxide Oxidisable Sulfur (23E)	----	0.020	% S		0.031	0.048	<0.020	<0.020	<0.020
acidity - Peroxide Oxidisable Sulfur (a-23E)	----	10	mole H+ / t		20	30	12	<10	<10
EA029-D: Calcium Values									
KCl Extractable Calcium (23Vh)	----	0.020	% Ca		0.037	<0.020	<0.020	0.129	0.022
Peroxide Calcium (23Wh)	----	0.020	% Ca		0.037	<0.020	<0.020	0.129	0.025
Acid Reacted Calcium (23X)	----	0.020	% Ca		<0.020	<0.020	<0.020	<0.020	<0.020
acidity - Acid Reacted Calcium (a-23X)	----	10	mole H+ / t		<10	<10	<10	<10	<10
sulfidic - Acid Reacted Calcium (s-23X)	----	0.020	% S		<0.020	<0.020	<0.020	<0.020	<0.020
EA029-E: Magnesium Values									
KCl Extractable Magnesium (23Sm)	----	0.020	% Mg		0.165	0.071	0.083	0.182	0.107
Peroxide Magnesium (23Tm)	----	0.020	% Mg		0.165	0.075	0.087	0.182	0.108
Acid Reacted Magnesium (23U)	----	0.020	% Mg		<0.020	<0.020	<0.020	<0.020	<0.020
Acidity - Acid Reacted Magnesium (a-23U)	----	10	mole H+ / t		<10	<10	<10	<10	<10
sulfidic - Acid Reacted Magnesium (s-23U)	----	0.020	% S		<0.020	<0.020	<0.020	<0.020	<0.020
EA029-G: Retained Acidity									
HCl Extractable Sulfur (20Be)	----	0.020	% S		----	----	0.042	----	----
Net Acid Soluble Sulfur (20Je)	----	0.020	% S		----	----	<0.020	----	----
acidity - Net Acid Soluble Sulfur (a-20J)	----	10	mole H+ / t		----	----	<10	----	----
sulfidic - Net Acid Soluble Sulfur (s-20J)	----	0.020	% pyrite S		----	----	<0.020	----	----



Analytical Results

Sub-Matrix: **SOIL**
 (Matrix: **SOIL**)

Client sample ID

				30615/ASS3	30615/ASS4	30615/ASS5	30615/ASS6	30615/ASS7
Client sampling date / time				01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00
Compound	CAS Number	LOR	Unit	ES2034713-021	ES2034713-022	ES2034713-023	ES2034713-024	ES2034713-025
				Result	Result	Result	Result	Result
EA029-H: Acid Base Accounting								
ANC Fineness Factor	----	0.5	-	1.5	1.5	1.5	1.5	1.5
Net Acidity (sulfur units)	----	0.02	% S	0.07	0.12	0.22	0.02	0.05
Net Acidity (acidity units)	----	10	mole H+ / t	45	78	137	10	33
Liming Rate	----	1	kg CaCO3/t	3	6	10	1	2
Net Acidity excluding ANC (sulfur units)	----	0.02	% S	0.07	0.12	0.22	0.02	0.05
Net Acidity excluding ANC (acidity units)	----	10	mole H+ / t	45	78	137	10	33
Liming Rate excluding ANC	----	1	kg CaCO3/t	3	6	10	1	2

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30615/ASS8	30616/S1	30616/S2	30616/S3	30616/ASS1
Client sampling date / time				01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00
Compound	CAS Number	LOR	Unit	ES2034713-026	ES2034713-027	ES2034713-028	ES2034713-029	ES2034713-030	
				Result	Result	Result	Result	Result	
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit	----	5.5	6.0	6.1	----	
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm	----	41	32	27	----	
EA029-A: pH Measurements									
pH KCl (23A)	----	0.1	pH Unit	5.2	----	----	----	5.2	
pH OX (23B)	----	0.1	pH Unit	4.9	----	----	----	3.8	
EA029-B: Acidity Trail									
Titratable Actual Acidity (23F)	----	2	mole H+ / t	10	----	----	----	10	
Titratable Peroxide Acidity (23G)	----	2	mole H+ / t	42	----	----	----	15	
Titratable Sulfidic Acidity (23H)	----	2	mole H+ / t	31	----	----	----	5	
sulfidic - Titratable Actual Acidity (s-23F)	----	0.020	% pyrite S	<0.020	----	----	----	<0.020	
sulfidic - Titratable Peroxide Acidity (s-23G)	----	0.020	% pyrite S	0.067	----	----	----	0.024	
sulfidic - Titratable Sulfidic Acidity (s-23H)	----	0.020	% pyrite S	0.050	----	----	----	<0.020	
EA029-C: Sulfur Trail									
KCl Extractable Sulfur (23Ce)	----	0.020	% S	<0.020	----	----	----	<0.020	
Peroxide Sulfur (23De)	----	0.020	% S	0.051	----	----	----	0.025	
Peroxide Oxidisable Sulfur (23E)	----	0.020	% S	0.051	----	----	----	0.025	
acidity - Peroxide Oxidisable Sulfur (a-23E)	----	10	mole H+ / t	32	----	----	----	16	
EA029-D: Calcium Values									
KCl Extractable Calcium (23Vh)	----	0.020	% Ca	0.053	----	----	----	0.123	
Peroxide Calcium (23Wh)	----	0.020	% Ca	0.064	----	----	----	0.123	
Acid Reacted Calcium (23X)	----	0.020	% Ca	<0.020	----	----	----	<0.020	
acidity - Acid Reacted Calcium (a-23X)	----	10	mole H+ / t	<10	----	----	----	<10	
sulfidic - Acid Reacted Calcium (s-23X)	----	0.020	% S	<0.020	----	----	----	<0.020	
EA029-E: Magnesium Values									
KCl Extractable Magnesium (23Sm)	----	0.020	% Mg	0.066	----	----	----	0.049	
Peroxide Magnesium (23Tm)	----	0.020	% Mg	0.068	----	----	----	0.049	
Acid Reacted Magnesium (23U)	----	0.020	% Mg	<0.020	----	----	----	<0.020	
Acidity - Acid Reacted Magnesium (a-23U)	----	10	mole H+ / t	<10	----	----	----	<10	
sulfidic - Acid Reacted Magnesium (s-23U)	----	0.020	% S	<0.020	----	----	----	<0.020	
EA029-H: Acid Base Accounting									



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30615/ASS8	30616/S1	30616/S2	30616/S3	30616/ASS1
Client sampling date / time					01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00
Compound	CAS Number	LOR	Unit		ES2034713-026	ES2034713-027	ES2034713-028	ES2034713-029	ES2034713-030
					Result	Result	Result	Result	Result
EA029-H: Acid Base Accounting - Continued									
ANC Fineness Factor	----	0.5	-		1.5	----	----	----	1.5
Net Acidity (sulfur units)	----	0.02	% S		0.07	----	----	----	0.04
Net Acidity (acidity units)	----	10	mole H+ / t		42	----	----	----	26
Liming Rate	----	1	kg CaCO3/t		3	----	----	----	2
Net Acidity excluding ANC (sulfur units)	----	0.02	% S		0.07	----	----	----	0.04
Net Acidity excluding ANC (acidity units)	----	10	mole H+ / t		42	----	----	----	26
Liming Rate excluding ANC	----	1	kg CaCO3/t		3	----	----	----	2
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	0.1	%		----	14.6	8.6	8.0	----
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		----	20	<10	<10	----
ED045G: Chloride by Discrete Analyser									
Chloride	16887-00-6	10	mg/kg		----	20	<10	<10	----



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30616/ASS2	30616/ASS3	30616/ASS4	----	----
Client sampling date / time					01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	----	----
Compound	CAS Number	LOR	Unit		ES2034713-031	ES2034713-032	ES2034713-033	-----	-----
				Result	Result	Result		----	----
EA029-A: pH Measurements									
pH KCl (23A)	----	0.1	pH Unit		5.1	5.3	5.2	----	----
pH OX (23B)	----	0.1	pH Unit		5.0	4.6	5.5	----	----
EA029-B: Acidity Trail									
Titrateable Actual Acidity (23F)	----	2	mole H+ / t		9	4	5	----	----
Titrateable Peroxide Acidity (23G)	----	2	mole H+ / t		10	4	7	----	----
Titrateable Sulfidic Acidity (23H)	----	2	mole H+ / t		<2	<2	<2	----	----
sulfidic - Titrateable Actual Acidity (s-23F)	----	0.020	% pyrite S		<0.020	<0.020	<0.020	----	----
sulfidic - Titrateable Peroxide Acidity (s-23G)	----	0.020	% pyrite S		<0.020	<0.020	<0.020	----	----
sulfidic - Titrateable Sulfidic Acidity (s-23H)	----	0.020	% pyrite S		<0.020	<0.020	<0.020	----	----
EA029-C: Sulfur Trail									
KCl Extractable Sulfur (23Ce)	----	0.020	% S		<0.020	<0.020	<0.020	----	----
Peroxide Sulfur (23De)	----	0.020	% S		<0.020	<0.020	<0.020	----	----
Peroxide Oxidisable Sulfur (23E)	----	0.020	% S		<0.020	<0.020	<0.020	----	----
acidity - Peroxide Oxidisable Sulfur (a-23E)	----	10	mole H+ / t		<10	<10	<10	----	----
EA029-D: Calcium Values									
KCl Extractable Calcium (23Vh)	----	0.020	% Ca		0.047	0.098	<0.020	----	----
Peroxide Calcium (23Wh)	----	0.020	% Ca		0.047	0.098	<0.020	----	----
Acid Reacted Calcium (23X)	----	0.020	% Ca		<0.020	<0.020	<0.020	----	----
acidity - Acid Reacted Calcium (a-23X)	----	10	mole H+ / t		<10	<10	<10	----	----
sulfidic - Acid Reacted Calcium (s-23X)	----	0.020	% S		<0.020	<0.020	<0.020	----	----
EA029-E: Magnesium Values									
KCl Extractable Magnesium (23Sm)	----	0.020	% Mg		0.062	0.037	0.111	----	----
Peroxide Magnesium (23Tm)	----	0.020	% Mg		0.063	0.039	0.113	----	----
Acid Reacted Magnesium (23U)	----	0.020	% Mg		<0.020	<0.020	<0.020	----	----
Acidity - Acid Reacted Magnesium (a-23U)	----	10	mole H+ / t		<10	<10	<10	----	----
sulfidic - Acid Reacted Magnesium (s-23U)	----	0.020	% S		<0.020	<0.020	<0.020	----	----
EA029-H: Acid Base Accounting									
ANC Fineness Factor	----	0.5	-		1.5	1.5	1.5	----	----
Net Acidity (sulfur units)	----	0.02	% S		<0.02	<0.02	<0.02	----	----
Net Acidity (acidity units)	----	10	mole H+ / t		<10	<10	<10	----	----
Liming Rate	----	1	kg CaCO3/t		1	<1	<1	----	----

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 Work Order : ES2034713
 Client : STS Geotechnics
 Project : 30055/30060/30614/30615/30616



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30616/ASS2	30616/ASS3	30616/ASS4	----	----
				Client sampling date / time	01-Oct-2020 00:00	01-Oct-2020 00:00	01-Oct-2020 00:00	----	----
Compound	CAS Number	LOR	Unit		ES2034713-031	ES2034713-032	ES2034713-033	-----	-----
					Result	Result	Result	----	----
EA029-H: Acid Base Accounting - Continued									
Net Acidity excluding ANC (sulfur units)	----	0.02	% S		<0.02	<0.02	<0.02	----	----
Net Acidity excluding ANC (acidity units)	----	10	mole H+ / t		<10	<10	<10	----	----
Liming Rate excluding ANC	----	1	kg CaCO3/t		1	<1	<1	----	----